

Determination of Local Tie Vectors

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Global Geodetic Reference Frames

- Aimed accuracy goal of 1 mm in position on a global scale
- Combination of several space-geodetic techniques
 - Global Navigation Satellite System (GNSS)
 - Doppler Orbitography and Radiopositioning Integrated by Satellite (DORIS)
 - Satellite Laser Ranging (SLR)
 - Very Long Baseline Interferometry (VLBI)
- Largely independent techniques
 - Pure combination yields unreliable results (weak physical connection)
 - Tying information is of crucial importance

Global Geodetic Reference Frames

- Global tying types
 - Tying via common Earth-orientation parameters (EOP) → Global ties
 - Tying via multi-technique satellites → Space ties
- Local tying types
 - Tying via geometric conditions → Local ties
 - Tying via common atmospheric parameters → Atmospheric ties
 - Tying via common clocks → Clock ties
 - Require multi-technique station operating at least two geodetic techniques

Local ties

- Observable at multi-technique station via terrestrial measurement instruments
 - Polar instruments (total station, laser tracker, ...)
 - Precise distance measurement instruments (DistriMetre, TeleYAG, ...)
 - Spirit levelling
 - Photogrammetric systems
 - GNSS
 - ...

Local ties

- Defined between invariant reference points of space-geodetic techniques
- Provided via SINEX file
- Consists of reference points and related fully populated dispersion matrix
- Relates to Global Geodetic Reference Frame

- Accuracy requirement < 1 mm
- Traceable to SI unit
- In-process determination approaches required to reduce telescope downtime
- Reference point determination approaches should be frame-independent

DORIS – Invariant Reference Point

- Reference point $\mathbf{p}_{\text{DORIS}}$ defined at beacon's main-axis
- 390 mm above reference plane
- Combination of cylinder and plane

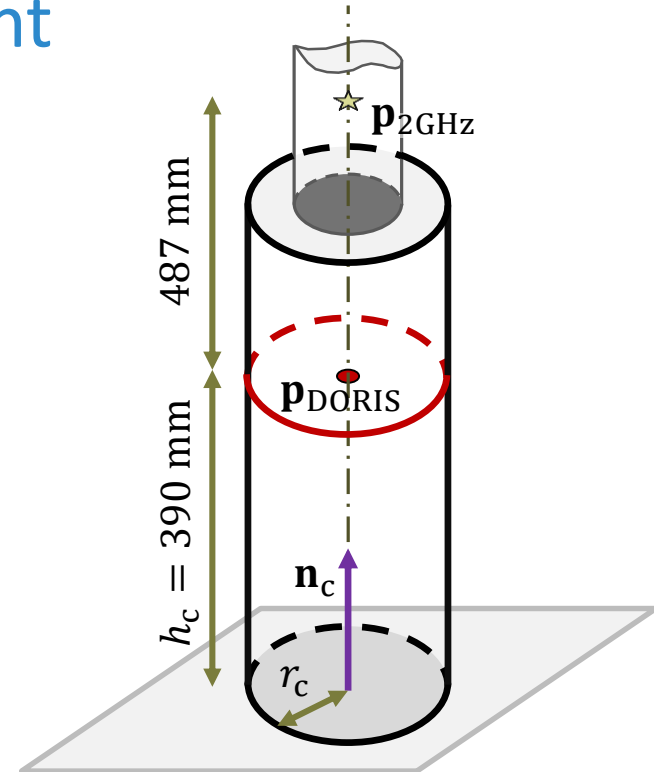
$$\|(\mathbf{p} - \mathbf{p}_{\text{DORIS}}) \times \mathbf{n}_c\|_2 = r_c$$

$$\mathbf{n}_c^T \mathbf{p} = d_c$$

with the restrictions

$$\mathbf{n}_c^T \mathbf{p}_{\text{DORIS}} - d_c = h_c$$

$$\|\mathbf{n}_c\|_2 = 1$$



GNSS – Invariant Reference Point

- Reference point \mathbf{p}_{GNSS} defined at antenna's main-axis at bottom of pre-amplifier
- Combination of sphere and plane

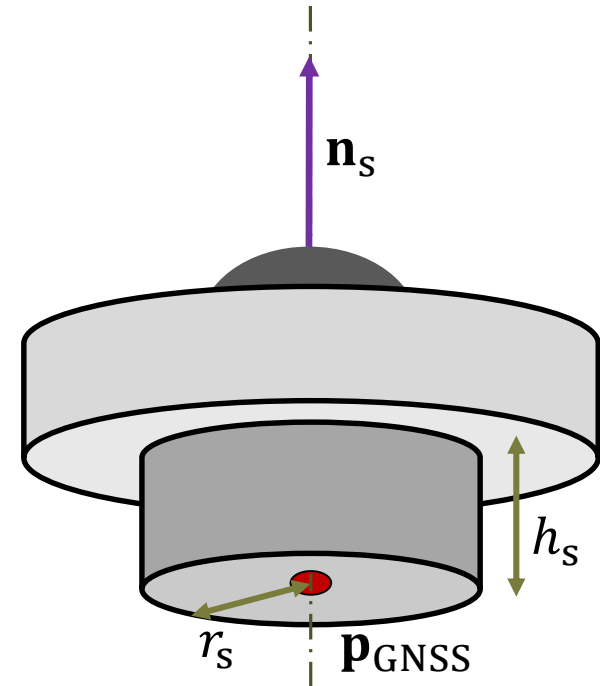
$$\|(\mathbf{p} - \mathbf{p}_{\text{GNSS}})\|_2 = r_s$$

$$\mathbf{n}_s^T \mathbf{p} = d_s$$

with the restrictions

$$\mathbf{n}_s^T \mathbf{p}_{\text{GNSS}} - d_s = -h_s$$

$$\|\mathbf{n}_s\|_2 = 1$$

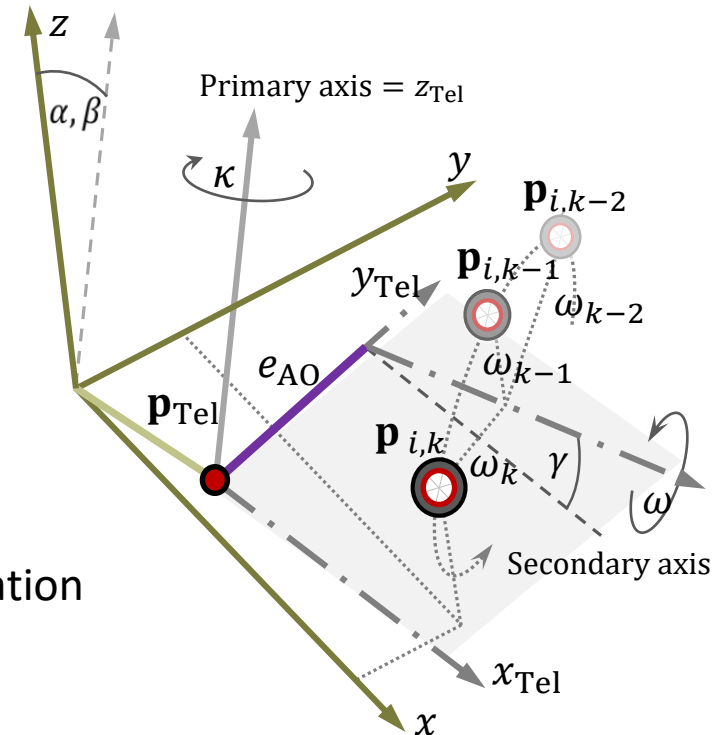


SLR/VLBI – Invariant Reference Point

- Reference point \mathbf{p}_{Tel} defined as orthogonal projection of secondary axis onto primary axis
- Transformation between telescope-fixed frame and Earth-fixed frame via

$$\mathbf{p}_{i,k} = \mathbf{p}_{\text{Tel}} + \mathbf{R}_x(\beta)\mathbf{R}_y(\alpha)\mathbf{R}_z^T(\kappa_k^*)\mathbf{R}_y(\gamma)(\mathbf{e}_{\text{AO}} + \mathbf{R}_x(\omega_k)\mathbf{q}_i)$$

- Allows for in-process reference point determination
- Downtime reduction



SAR – Invariant Reference Point

- Increasing use of Synthetic Aperture Radar (SAR)
- Reference point \mathbf{p}_{SAR} defined as intersection of three materialized planes (inner corner)

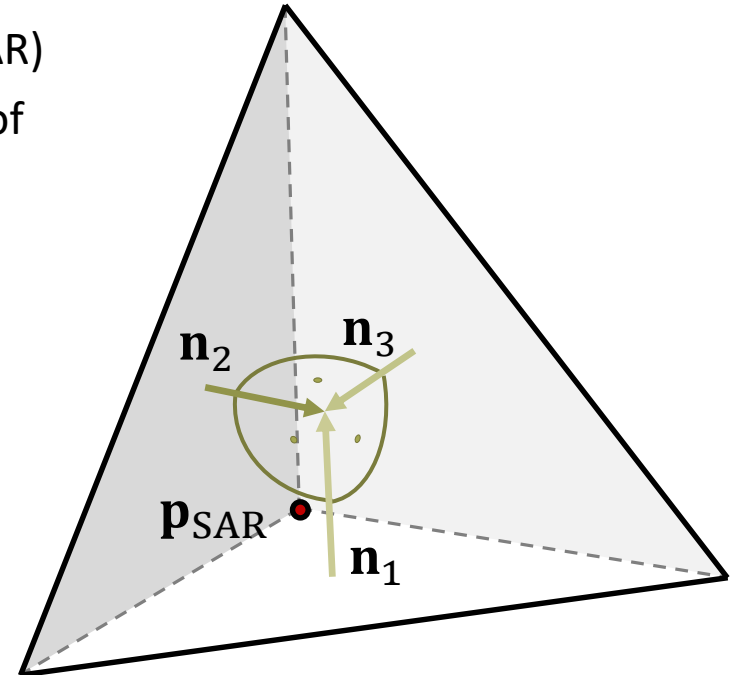
$$\mathbf{n}_j^T \mathbf{p} = d_j \quad \forall j = \{1,2,3\}$$

with the restrictions

$$\mathbf{n}_j^T \mathbf{p}_{\text{SAR}} = d_j$$

$$\|\mathbf{n}_j\|_2 = 1$$

$$\mathbf{n}_j^T \mathbf{n}_k = 0$$

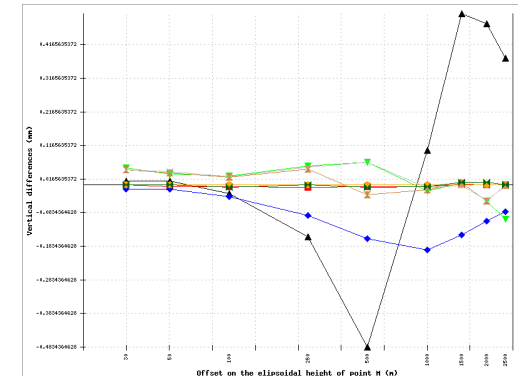
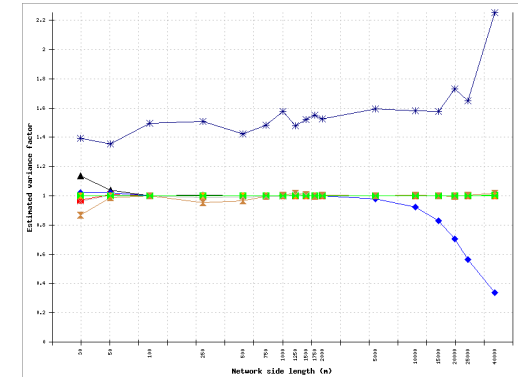


Network Adjustment

- Evaluation as spatial network is strongly recommended
 - Datum relates to Global Geodetic Reference Frame
 - Results refer directly to target frame
 - Implicit transformation of local side network to global frame
 - Analysis difficult and results hard to interpret
 - Datum relates to local (topocentric) frame
 - Results refer to an arbitrarily defined frame
 - Explicit transformation of local side network to global frame
 - Analysis is clear and results are easy to interpret
- Both procedures yield identical results

Network Adjustment

- Round robin test
 - Differences in implementation
 - Functional model
 - Stochastic model
 - Handling of auxiliary parameters
 - Operator software impact
- Adjustment results
 - Differences in parameters
 - Differences in dispersion matrix



Network Adjustment

- Handling of deflection of the vertical
 - Introducing as known (fixed) parameters (4DOF)
 - Treating as parameters to be estimated (6DOF)
- Adjustment results depend on realization of datum
- Misspecified datum yields network tilt and bending
- Risk of weak form increases for long stretched or large networks
- Network adjustment yields
 - Spatial coordinates of observed points collected in vector \mathbf{y}
 - Fully populated dispersion matrix $\Sigma_{\mathbf{yy}}$

Sequential Reference Point Determinations

- Estimate reference point \mathbf{x} using technique-specific observed points \mathbf{y}_*
- Remove technique-specific observed points \mathbf{y}_* from \mathbf{y}

$$\bar{\mathbf{y}} \leftarrow \mathbf{E}\mathbf{y}$$

- Join coordinates of reference point \mathbf{x} and (remaining) network points $\bar{\mathbf{y}}$

$$\mathbf{y}^T \leftarrow [\mathbf{x}^T \quad \bar{\mathbf{y}}^T]$$

- Obtain fully populated dispersion matrix $\Sigma_{\mathbf{y}\mathbf{y}}$ of \mathbf{x} and $\bar{\mathbf{y}}$

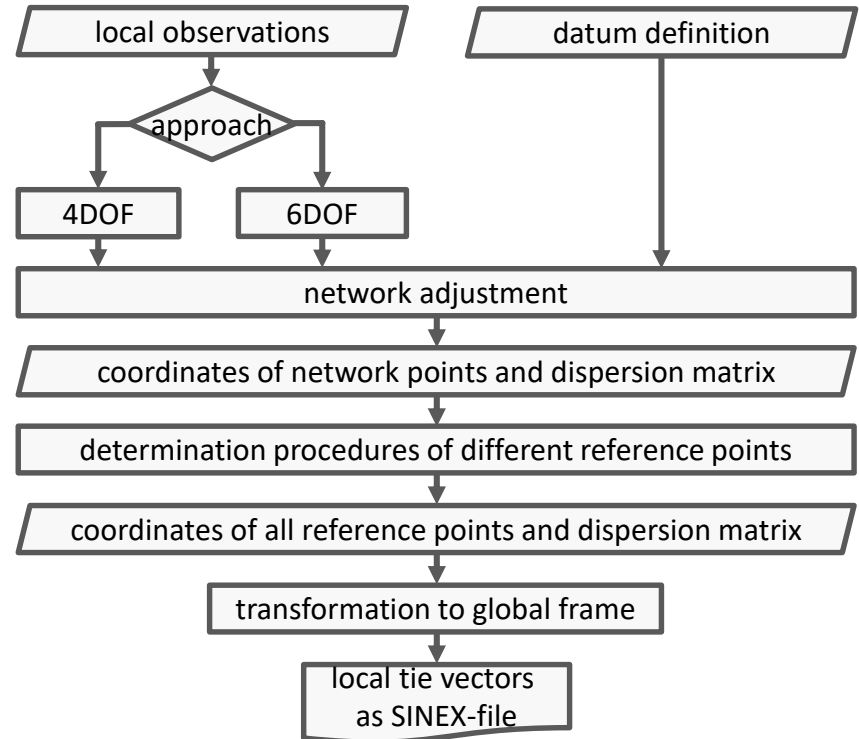
$$\Sigma_{\mathbf{y}\mathbf{y}} \leftarrow \begin{bmatrix} \Sigma_{\mathbf{x}\mathbf{x}} & \Sigma_{\mathbf{x}\bar{\mathbf{y}}} \\ \Sigma_{\bar{\mathbf{y}}\mathbf{x}} & \Sigma_{\bar{\mathbf{y}}\bar{\mathbf{y}}} \end{bmatrix}$$

- Repeat procedure until all reference points are determined
- In final step \mathbf{y} contains all reference points and $\Sigma_{\mathbf{y}\mathbf{y}}$ is fully populated

Conclusion

- Precise datum definition
- Reliable measurements
- Uncertainty budgeting
- Network adjustment
- Reference point determination
- Rigorous uncertainty propagation

→ Local tie determination is challenging metrological task



Thank you for your attention!



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Funding

This project 18SIB01 GeoMetre has received funding from the EMPIR programme co-financed by the Participating States and from the European Union's Horizon 2020 research and innovation programme.